DESIGN CRITERIA

1. DESIGN GRAVITY LIVE LOADS:

AREA	LIVE LOAD
SLAB–ON–GRADE	100 PSF
ROOF	30 PSF

2. SNOW LOADING IBC 2003

GROUND SNOW LOAD	25 PSF
FLAT ROOF SNOW LOAD	20 PSF + SNOW DRIFT
SNOW EXPOSURE FACTOR	0.9
SNOW IMPORTANCE FACTOR	1.0
THERMAL FACTOR	1.0

3. WIND LOADING IBC 2003

BASIC WIND SPEED (3 SECOND GUST)	90 MPH
WIND LOAD IMPORTANCE FACTOR	1.0
WIND EXPOSURE CATEGORY	B
INTERNAL PRESSURE COEFFICIENT	0.25
WIND DESIGN PRESSURE	20 PSF
COMPONENT AND CLADDING PRESSURE	29 PSF

4. SEISMIC LOADING IBC 2003

SEISMIC USE GROUP	II
Ss	0.187 G
S1	0.063 G
SDS	0.150 G
SD1	0.071 G
SITE CLASS	D
SEISMIC IMPORTANCE FACTOR	1.15
SEISMIC DESIGN CATEGORY	B
BASIC SEISMIC FORCE RESISTING SYSTEM:	ORDINARY REINFORCED MASONRY SHEAR WALLS

5. ANY MECHANICAL EQUIPMENT NOT SHOWN ON THE STRUCTURAL DRAWINGS AND HAVING A WEIGHT IN EXCESS OF 500 POUNDS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION.

SUBMITTALS

1. BEFORE SUBMISSION OF SHOP DRAWINGS, THE CONTRACTOR SHALL HAVE DETERMINED AND VERIFIED ALL QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS AND SIMILAR DATA AND SHALL HAVE COORDINATED EACH SHOP DRAWING WITH OTHER SHOP DRAWINGS AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
2. PRIOR TO SUBMISSIONS, THE CONTRACTOR SHALL STAMP OR PROVIDE A SIMILAR WRITTEN INDICATION THAT THE CONTRACTOR HAS REVIEWED THE SUBMISSION AND IS SATISFIED THE CONTENTS ARE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.
3. REPRINTS OF THE CONTRACT DOCUMENTS WILL NOT BE ACCEPTED.
4. NO DIMENSIONAL INFORMATION MAY BE OBTAINED BY DIRECT SCALING OF THE DRAWINGS.

EXISTING CONSTRUCTION

1. ALL MEMBER SIZES AND DIMENSIONS AND ELEVATIONS OF EXISTING STRUCTURES SHOWN ON THE DRAWINGS ARE OBTAINED FROM AVAILABLE SOURCES, AND ARE NOT GUARANTEED TO BE TRUE AND EXACT. THE CONTRACTOR SHALL VERIFY THESE DIMENSIONS AND ELEVATIONS BY ACTUAL FIELD MEASUREMENTS PRIOR TO FABRICATION OF ANY MATERIALS AND START OF WORK, AND REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER.
2. THE CONTRACTOR SHALL PROVIDE ALL SHORING, NEEDLING AND BRACING AS REQUIRED TO SUPPORT THE EXISTING STRUCTURE. THE CONTRACTOR SHALL EXAMINE THE EXISTING STRUCTURE TO DETERMINE THE EXTENT OF NECESSARY SHORING, NEEDLING AND UNDERPINNING. THE CAPACITY AND METHOD USED FOR SHORING AND NEEDLING SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

DEMOLITION NOTES

1. REMOVE EXISTING CONSTRUCTION AS SHOWN ON PLANS. SEE PLANS, SECTIONS, AND DETAILS FOR EXTENT OF STRUCTURE TO BE REMOVED.
2. EXISTING STRUCTURAL FRAMING SHALL REMAIN UNLESS SPECIFICALLY NOTED ON PLAN TO BE REMOVED.
3. IF FIELD CONDITIONS DIFFER FROM THOSE SHOWN ON DRAWINGS, NOTIFY STRUCTURAL ARCHITECT/ENGINEER BEFORE PROCEEDING WITH DEMOLITION.
4. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO EXAMINE THE EXISTING BUILDING DURING THE COURSE OF CONSTRUCTION AND IMMEDIATELY ADVISE THE ARCHITECT/ENGINEER OF ANY AREAS WHERE THE STRUCTURE EXHIBITS DISTRESS OR FAILURE.
5. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SATISFY HIMSELF AS TO THE LOCATION OF ANY EXISTING SYSTEMS IN THE IMMEDIATE VICINITY OF CONSTRUCTION SO AS TO PREVENT DAMAGE TO THEM. SHOULD ANY DAMAGE TO SUCH SYSTEMS OCCUR THE CONTRACTOR SHALL BE REQUIRED TO REPAIR SUCH DAMAGE AT HIS OWN EXPENSE AND TO THE SATISFACTION OF THE OWNER.

FOUNDATIONS

1. ALL SPREAD FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR CONTROLLED STRUCTURAL FILL, HAVING A MINIMUM SAFE BEARING CAPACITY OF 2,000 PSF. ALL SPREAD FOOTINGS SHALL PROJECT AT LEAST 1'-0" INTO SOIL HAVING SUCH MINIMUM BEARING VALUE.
2. RETAIN THE SERVICES OF A REGISTERED GEOTECHNICAL ENGINEER, APPROVED BY THE OWNER, TO VERIFY SOIL BEARING CAPACITY AT EACH FOOTING PRIOR TO INSTALLATION. NOTIFY ARCHITECT/ENGINEER OF ANY VARIATION FROM ANTICIPATED BEARING CAPACITY FOR APPROPRIATE REDESIGN OR LOWERING OF FOOTINGS.
3. EXCAVATION, PREPARATION OF SUBGRADE, AND FOOTING CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT BY D.W. KOZERA, INC. AND DATED FEBRUARY 6, 2007.
4. COMPACT FILL AND BACKFILL TO 95% OF A.A.S.H.T.O. T–180. ALL FILL AND BACKFILL OPERATIONS SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER.
5. ALL ORGANIC MATERIALS AND CONSTRUCTION DEBRIS SHALL BE REMOVED IN REGIONS OF ALL FOUNDATIONS.
6. THE BOTTOMS OF ALL EXTERIOR FOOTINGS SHALL BE 2'–6" MINIMUM BELOW FINISHED GRADE.
7. EDGES OF FOOTINGS SHALL NOT BE PLACED AT A GREATER THAN 1 (VERTICAL) TO 2 (HORIZONTAL) SLOPE WITH RESPECT TO ANY ADJACENT FOOTING OR EXCAVATION.
8. THE CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS, AND ALL EXCAVATIONS SHALL BE KEPT FREE OF WATER.
9. NO HORIZONTAL JOINTS SHALL BE PLACED IN WALLS EXCEPT AS SHOWN ON THE DRAWINGS WITHOUT APPROVAL OF THE ARCHITECT/ENGINEER.
10. THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS FOR ALL LOCATIONS OF TRENCHES, PITS, CONDUITS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

FOUNDATION CONCRETE

1. ALL CONCRETE SHALL CONFORM TO THE PROVISIONS OF ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318–LATEST EDITION) AND ACI SPECIFICATIONS FOR STRUCTURAL CONCRETE IN BUILDINGS (ACI 301–LATEST EDITION).
2. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE HAVING A DESIGN COMPRESSIVE STRENGTH AT 28 DAYS AS FOLLOWS:

A. FOOTINGS	3,000 PSI
B. SLAB ON GRADE	3,000 PSI

3. NO CONCRETE SHALL BE PLACED UNTIL CONCRETE DESIGN MIXES HAVE BEEN SUBMITTED FOR EACH CLASS OF CONCRETE NOTED ABOVE AND HAVE BEEN REVIEWED BY THE ARCHITECT/ENGINEER.
4. USE A WATER REDUCING ADMIXTURE IN ALL CONCRETE.
5. USE A MINIMUM OF 5 1/2 BAGS OF CEMENT FOR EACH CUBIC YARD OF CONCRETE.
6. SLUMP SHALL BE AS REQUIRED BY ACI 302 – LATEST EDITION
7. AIR ENTRAIN ALL CONCRETE EXPOSED TO WEATHER TO 5%±1%.
8. NO CALCIUM CHLORIDE IN ANY FORM WILL BE PERMITTED IN CONCRETE.
9. ALL STRUCTURAL MEMBERS SHALL BE POURED FOR THEIR FULL DEPTHS IN ONE OPERATION.
10. EXCAVATIONS SHALL BE KEPT FREE OF WATER. NO CONCRETE SHALL BE PLACED IN WATER.
11. ALL SLABS ON GRADE SHALL HAVE THICKENINGS, DEPRESSIONS, OPENINGS, ETC. AS SHOWN OR AS REQUIRED BY VARIOUS TRADES.
12. REFER TO ARCHITECTURAL DRAWINGS AND/OR SPECIFICATION SECTIONS FOR CONCRETE FINISHES.
13. RETAIN THE SERVICES OF AN INDEPENDENT TESTING AGENCY APPROVED BY THE OWNER TO PERFORM TESTS OF CONCRETE. REFER TO THE PROJECT SPECIFICATIONS.

REINFORCEMENT

1. ALL DEVELOPMENT AND SPLICES OF REINFORCEMENT SHALL CONFORM TO THE PROVISIONS OF ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318–LATEST EDITION).
2. REINFORCING STEEL SHALL BE DEFORMED BARS OF INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO CURRENT REQUIREMENTS OF ASTM A615 GRADE 60 EXCEPT TIES MAY BE GRADE 40. ALL HOOKS SHALL BE STANDARD HOOKS, UNLESS OTHERWISE NOTED.
3. WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A185 (LATEST EDITION).
4. ALL WELDED WIRE FABRIC SHALL BE SPLICED SO THAT THE OVERLAP OF THE OUTERMOST CROSS WIRES OF EACH ADJOINING SHEET IS NOT LESS THAN THE SPACING OF THE CROSS WIRES PLUS TWO INCHES, UNLESS NOTED OTHERWISE.
5. REINFORCING BAR SUPPORTS AND SPACERS SHALL CONFORM TO ACI 315–(LATEST EDITION) DETAILING MANUAL.
6. SHOP DRAWINGS SHOWING ALL NECESSARY SECTIONS AND DETAILS FOR THE PROPER POSITIONING OF ALL REINFORCING STEEL SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW BEFORE FABRICATION OR PLACEMENT OF THE STEEL.

CONCRETE MASONRY

1. CONCRETE MASONRY CONSTRUCTION SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530–LATEST EDITION) AND ACI SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1–LATEST EDITION).
2. CONCRETE MASONRY UNITS (CMU) SHALL BE GRADE N, 2–CELL BLOCK WITH A MINIMUM COMPRESSIVE STRENGTH F'm=2500 PSI ON NET CROSS SECTION UNLESS OTHERWISE NOTED. CMU SHALL BE TYPE I FOR EXTERIOR AND FOUNDATION WALLS. TYPE I OR TYPE II MAY BE USED FOR INTERIOR PARTITION WALLS.
3. CONCRETE MASONRY UNITS SHALL CONFORM TO THE REQUIREMENTS OF THE MOST RECENT EDITIONS OF THE FOLLOWING STANDARDS:

A. HOLLOW LOAD–BEARING UNITS	ASTM C90
B. SOLID LOAD–BEARING UNITS	ASTM C145
C. HOLLOW NON–LOAD–BEARING UNITS	ASTM C129
D. CONCRETE BUILDING BRICK	ASTM C55

4. ALL CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT.
5. MORTAR FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO THE REQUIREMENTS OF ASTM C270 (LATEST EDITION), TYPE S.
6. GROUT FOR REINFORCED OR NONREINFORCED MASONRY SHALL CONFORM TO THE REQUIREMENTS OF ASTM C476 (LATEST EDITION).
7. WHERE DRAWINGS INDICATE CELLS OF CONCRETE MASONRY UNITS (CMU) ARE TO BE FILLED SOLID, CELLS OF CMU SHALL BE FILLED WITH 3000 PSI PEA GRAVEL CONCRETE IN SIX COURSE MAXIMUM LIFTS.
8. WHERE DRAWINGS SPECIFY GROUT TO BE PROVIDED, 3000 PSI PEA GRAVEL CONCRETE CAN BE SUBSTITUTED IF APPLICABLE TO USE (3000 PSI PEA GRAVEL CONCRETE SHALL NOT BE USED UNDER COLUMN BASEPLATES OR BEARING PLATES).
9. ALL REINFORCEMENT SHOWN IN WALLS SHALL BE CENTERED IN MASONRY UNITS UNLESS NOTED OTHERWISE.
10. ALL BLOCK SHALL HAVE GALVANIZED, TRUSS OR LADDER TYPE, HORIZONTAL JOINT REINFORCEMENT AT 16" O/C MAXIMUM WITH PREFABRICATED CORNER AND "T" PIECES UNLESS NOTED OTHERWISE. LAP ALL SPLICES 6" MINIMUM. PROVIDE AN ADDITIONAL ROW ABOVE AND BELOW ALL OPENINGS AND EXTEND TWO FEET BEYOND JAMBS. STOP HORIZONTAL REINFORCING EACH SIDE OF CONTROL JOINTS.
11. WHERE MASONRY WALLS OF HOLLOW UNITS OR MASONRY BONDED WALLS CHANGE IN THICKNESS, THE WALL SHALL BE FILLED SOLID WITH CONCRETE OR OTHERWISE CONSTRUCTED SOLID FOR AT LEAST ONE COURSE (8" MIN) IMMEDIATELY BELOW SUCH LEVEL WHERE THE THICKNESS CHANGES.
12. EXCEPT AS OTHERWISE SHOWN, CELLS IN MASONRY UNDER BEARING AREAS FOR BEAMS, LINTELS, AND SLABS SHALL BE FILLED SOLID WITH CONCRETE FOR AT LEAST TWO COURSES (16" MIN) IMMEDIATELY BELOW SUCH BEARING.
13. ALL MASONRY WALLS SHALL HAVE TEMPORARY BRACING INSTALLED UNTIL ALL STEEL AND DECK IS COMPLETE. CONTRACTOR IS SOLELY RESPONSIBLE FOR TEMPORARY BRACING.

STRUCTURAL STEEL SHOP DRAWINGS

1. SHOP DRAWINGS SHOWING ALL OF THE SECTIONS AND DETAILS NECESSARY FOR THE PROPER PLACEMENT AND CONNECTION OF ALL STRUCTURAL STEEL, STEEL JOISTS AND JOIST GIRDERS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW AND COMMENT PRIOR TO FABRICATION AND ERECTION.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL SHALL CONFORM TO THE AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS
2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS:
   A. STRUCTURAL STEEL W–SHAPES – A992 HAVING A MINIMUM YIELD STRENGTH OF 50 KSI.
   B. STRUCTURAL STEEL CHANNELS, ANGLES, BARS & PLATES – A36 HAVING A MINIMUM YIELD STRENGTH OF 36 KSI
3. BOLTS SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS: A. HIGH STRENGTH BOLTS – A325 B. ANCHOR BOLTS – A307 A36.
4. ALL BOLTS SHALL BE 3/4" DIAMETER, OPEN HOLES 13/16" DIAMETER, UNLESS OTHERWISE SHOWN OR NOTED.
5. WELDING SHALL BE IN ACCORDANCE WITH AWS CODE FOR WELDING IN BUILDING CONSTRUCTION (AWS D1.1) AND SHALL BE PERFORMED BY CERTIFIED WELDERS. ALL WELDS SHALL BE MADE WITH AWS A5.1 E–70XX ELECTRODES.
6. ALL SHOP CONNECTIONS SHALL BE HIGH STRENGTH BOLTED OR WELDED.
7. ALL FIELD CONNECTIONS SHALL BE HIGH STRENGTH BOLTED EXCEPT WHERE DETAILS INDICATE WELDING.
8. NO PENETRATIONS ARE PERMITTED THROUGH STRUCTURAL STEEL MEMBERS UNLESS INDICATED ON STRUCTURAL DRAWINGS OR APPROVED BY ARCHITECT/ENGINEER.
9. APPROVAL OF THE ARCHITECT/ENGINEER SHALL BE MANDATORY FOR THE USE OF CUTTING TORCH IN THE FIELD.
10. ALL GROUT UNDER STEEL PLATES SHALL BE NON–SHRINK "PRE–MIX" TYPE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.
11. FOR ALL MISCELLANEOUS STEEL CONSTRUCTION NOT SHOWN ON STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL AND MECHANICAL DRAWINGS.

STEEL JOISTS

1. OPEN WEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS AND CODE OF STANDARD PRACTICE AND SHALL BE FABRICATED BY A SJI MEMBER COMPANY.
2. THE ENDS OF STEEL JOISTS SHALL EXTEND A MINIMUM DISTANCE OF 2–1/2 INCHES OVER STEEL SUPPORT AND 4 INCHES OVER ALL OTHER SUPPORTS. FASTEN ENDS BY BOLTING OR WELDING.
3. PROVIDE AND INSTALL HORIZONTAL AND/OR DIAGONAL BRIDGING AS SHOWN ON THE PLANS AND IN ACCORDANCE WITH SJI'S RECOMMENDATIONS. ANCHORS AT EACH LINE OF BRIDGING SHALL BE PROVIDED. BRIDGING SHALL BE WELDED TO JOISTS AND BEAMS AND ANCHORED TO MASONRY AS INDICATED. END BAYS OF EACH JOIST RUN SHALL HAVE BOTH HORIZONTAL AND DIAGONAL BRIDGING UNLESS NOTED OTHERWISE.
4. ALL BRIDGING SHALL BE IN PLACE AND ANCHORED BEFORE DECK INSTALLATION.
5. NO CONSTRUCTION LOADS SHALL BE PLACED ON THE JOISTS AND/OR JOIST GIRDERS BEFORE THEY ARE PERMANENTLY FASTENED TO SUPPORTS AND ALL BRIDGING AND ANCHORS ARE COMPLETELY INSTALLED. ALL STRUT JOIST CONNECTIONS (SHOWN S.J. ON PLAN) SHALL BE INSTALLED AFTER THE INITIAL BUILDING DEAD LOADS (SUCH AS ROOFING OR CONCRETE FLOORS) HAVE BEEN PLACED.

LINTELS

1. PROVIDE LINTELS OVER ALL PENETRATIONS IN MASONRY WALLS AT DOORS, WINDOWS, MECHANICAL AND ELECTRICAL SERVICES AND EQUIPMENT, ETC., AND AT LOCATIONS NOT SPECIFICALLY SHOWN ON THE DRAWINGS, IN ACCORDANCE WITH THE LINTEL SCHEDULE.
2. LINTELS IN INTERIOR, NON–LOAD BEARING CONCRETE MASONRY WALLS SHALL BE PRECAST CONCRETE TO MATCH THE TEXTURE, APPEARANCE AND THICKNESS OF THE WALLS OR PARTITIONS IN WHICH THEY ARE PLACED. PROVIDE PRECAST CONCRETE LINTELS PER PRECAST LINTEL SCHEDULE UNLESS OTHERWISE INDICATED.
3. CONTRACTOR SHALL SHORE ALL LINTELS AS REQUIRED TO PREVENT ROTATION DURING CONSTRUCTION.
4. CONTRACTOR SHALL COORDINATE SIZE AND TYPE OF LINTEL WITH ARCHITECTURAL, PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS.
5. CONTRACTOR SHALL PROVIDE SHORING AND/OR BRACING OF EXISTING WALLS AS REQUIRED TO INSTALL NEW LINTELS. METHOD OF SHORING AND/OR BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
6. ALL LINTELS TO HAVE SOLID BEARING, 8" MINIMUM, EACH END.
7. ALL LINTELS LOCATED IN EXTERIOR WALLS SHALL BE GALVANIZED.

LINTEL SCHEDULE

MARK	SIZE	COMMENTS
L–1	L4 X 3–1/2 X 5/16 LLV FOR EACH 4" OF WALL THICKNESS. UP TO 6'–0" OPENINGS	FOR CAVITY WALLS, USE 1–L6 X 6 X 5/16 LLV FOR VENEER.
L–2	L6 X 3–1/2 X 5/16 LLV FOR EACH 4" OF WALL THICKNESS. 6'–1" TO 8'–0" OPENINGS	FOR CAVITY WALLS, USE 1–L6 X 6 X 5/16 LLV FOR VENEER.
L–3	W8X31 WITH SUSPENDED PLATE. 8'–1" TO 10'–0" OPENINGS.	
P–1	1–4 X 8 PRECAST LINTEL WITH 1–#4 BOTTOM BAR FOR EACH 4" OF WALL THICKNESS. UP TO 2'–8" OPENINGS.	FOR CAVITY WALLS, USE 1–6 X 8 PRECAST LINTEL W/ 1–#4 BOTTOM BAR FOR VENEER
P–2	1–4 X 8 PRECAST LINTEL WITH 1–#3 TOP & 1–#4 BOTTOM BAR FOR EACH 4" OF WALL THICKNESS. 2'–9" TO 6'–8" OPENINGS.	FOR CAVITY WALLS, USE 1–6 X 8 PRECAST LINTEL WITH 1–#4 TOP AND BOTTOM BAR FOR VENEER.
P–3	1–4 X 8 PRECAST LINTEL WITH 1–#3 TOP & 1–#5 BOTTOM BAR FOR EACH 4" OF WALL THICKNESS. 6'–9" TO 8'–8" OPENINGS.	FOR CAVITY WALLS, USE 1–6 X 8 PRECAST LINTEL WITH 1–#5 TOP AND BOTTOM BAR FOR VENEER.

JOIST BEARING PLATE SCHEDULE

JOIST TYPE	PLATE SIZE	ANCHOR BOLTS
STANDARD 'K' JOISTS	4"x5/8"x0'–6"	(2) 1/2" DIA x 6" LONG HEADED STUDS
FILL CMU SOLID W/ 3,000 PSI CONCRETE AT BEARING LOCATIONS		

STRUCTURAL ABBREVIATIONS

AFF	ABOVE FINISHED FLOOR
ARCH	ARCHITECTURAL DRAWINGS
BFF	BELOW FINISHED FLOOR
BOD	BOTTOM OF DECK
B/	BOTTOM OF
BRG	BEARING
CJ	CONTROL JOINT
COORD	COORDINATE
DTL	DETAIL
DWG	DRAWING
EJ	EXPANSION JOINT
ELEV	ELEVATION
EXIST	EXISTING
EM	CALCULATED END MOMENT FOR WHICH MEMBER AND CONNECTION SHALL BE DESINGED
EOS	EDGE OF SLAB
FTG	FOOTING
IJ	ISOLATION JOINT
MANUF	MANUFACTURER
MB	MASONRY BEARING
MC	MOMENT RESISTING CONNECTION. MOMENT VALUE (IF STATED) IS IN UITS OF FT–KIPS. SEE TYPICAL DETAIL MCJ
OC	ON CENTER
R	CALCULATED END REACTION FOR WHICH THE CONNECTION SHALL BE DESIGNED
RTU	ROOFTOP UNIT
SEE	FOR ADDITONAL CONSTRUCTION REQUIREMENTS, REFER TO
SJ	STRUT JOIST
STD	STANDARD
T/	TOP OF
T/BRG	TOP OF STEEL BEARING PLATE
T/S	TOP OF STEEL
TDS	TURN–DOWN SLAB
T/F	TOP OF FOOTING
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VIF	VERIFY IN FIELD

ENGINEERS CERTIFICATION STATEMENT

I certify that these documents were prepared or approved by me, and that I am a duly licensed engineer under the law of the State of Maryland, license number 14446 , expiration date, 05/25/09.

BUREAU OF BUILDING SERVICES
APPROVED:_____ CHIEF
DATE:_____

APPROVED:_____ CHIEF
DATE:_____

APPROVED:_____ CHIEF
DATE:_____

AS-BUILT PER RECORD PRINT		REVISION	DATE	BY	P.W.A. NO.	KEY SHEETS	POSITION SHEETS	DRAWING SCALE
DRAFTSMAN	DATE							PLAN:
					RIGHT OF WAY			PROFILE: VERT.
								HOR.

ENGINEER:_____
DATE:_____ LIC.NO._____
SEAL

DESIGNED BY:_____
DRAWN BY:_____
CHECKED BY:_____

BUREAU OF ENGINEERING AND CONSTRUCTION	BUILDINGS	HIGHWAYS	STRUCTURES	STORM DRAINS	SEWER	WATER	FIELD ENGINEER
REVIEWED							
DATE							

DEPARTMENT OF PUBLIC WORKS
APPROVED:_____ DIRECTOR
DATE:_____

BUR. OF ENGINEERING & CONSTRUCTION
APPROVED:_____ CHIEF
DATE:_____

BALTIMORE COUNTY DEPARTMENT OF PUBLIC WORKS BUREAU OF ENGINEERING & CONSTRUCTION

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CCBC
The Community College of Baltimore County

BUILDING PERMIT # B664570

SHEET DESIGNATION	CONTRACT NO.
GENERAL NOTES	
	JOB ORDER NO.
	XXX-XX-XXX
	SHEET 27 OF 53
	DRAWING NO.
	S2-1
	FILE NO.

D–029135